

### **DIANA Seminar**

### Constitutive Model for the Non-linear Cyclic Behavior of Brick Masonry



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### **Overview**

### Background

Description of constitutive model

□Validation against experiments

Comparison with existing models

Conclusions



# Background



# Background



# Background

<u>TSRCM</u>



<u>EMM</u>



 $(\sigma_1, \sigma_2)$ 

- + Damage localization
- Capacity overestimation
- Energy dissipation



- $(\sigma_{xx}, \sigma_{yy}, \tau_{xy})$
- Damage localization
- + Capacity estimation
- + Energy dissipation

- Orthotropic Total-Strain-Rotating-Crack Model
- Orthotropic behavior
- Description of the second s
- □ Failure in tension, compression
- Tensile softening depending on cracking angle
- □ (Indirect) failure in shear
- Is independent material variables



# **TU**Delft

### Variation of material properties



Linear variation for  $E_{p,i}$ ,  $f_{c,i}$ ,  $G_{ft,i}$ ,  $G_{fc,i}$ 

**fu**Delft

After cracking: material properties are fixed to those corresponding to the cracking angle  $a_{crack}$ 







 $\beta_{i} = \begin{cases} \frac{\beta_{x}(|\alpha_{crack,i}| - \theta_{fl})^{2}}{\theta_{fl}^{2}} & \text{for } \theta_{fl} \ge |\alpha_{crack,i}| \ge 0^{o} \\ \beta_{y} \sin\left(4.5\left(|\alpha_{crack,i}| - (90^{o} - \theta_{fl})\right) & \text{for } 90^{o} \ge |\alpha_{crack,i}| \ge 90^{o} - \theta_{fl} \end{cases} \end{cases}$ 

### Cyclic behavior



### Shear limitation

- Coaxiality of  $\sigma_i \varepsilon_i$
- $\tau_{max} = \max\left(c_o, c_0 \tan\phi\left(\sigma_{yy0} + E_y \cdot \delta\varepsilon_{yy}\right)\right)$





### Validation

### TUD\_COMP\_4





#### Current model



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### Validation

### TUD\_COMP\_6

EMM



TSRCM



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#### **Current model**



E1 2.81e-09 2.00e-09 1.19e-09 3.79e-10 -4.30e-10 -1.24e-09 -2.05e-09 -3.67e-09

### TUD\_COMP\_4



#### **Current model**



#### TSRCM





#### Micro-IS-CCS



#### EMM





#### Micro-CCCS (Lourenco)



#### **Rankine Hill Anisotropy**



### TUD\_COMP\_6



**Current model** 







Micro-IS-CCS







#### Micro-CCCS (Lourenco)



**Rankine Hill Anisotropy** 



### HIGSTA





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### LOWSTA





Micro-IS-CCS

EMM



TSRCM



 **Rankine Hill Anisotropy** 





### **Computational time & effort**



	TUD-COMP-4	TUD-COMP-6	LOWSTA	HIGSTA
TSRCM	0:17:49	1:31:34	0:06:48	0:18:07
EMM	0:11:44	-	0:03:12	0:04:17
RHA	0:45:54	-	1:01:35	1:14:54
USRMAT	1:12:53	1:16:27	0:19:20	0:12:25
CCCS	0:18:30	1:04:54	0:09:05	0:11:56
SI-CCS	0:37:37	1:40:26	0:16:15	0:23:18

### Conclusions

□Accurate prediction of base shear capacity

Improvement of dissipated energy

□Sharp damage localization

Numerical instabilities

Computational time



# Thank you 🕲



### **Tensile Behaviour**

Tensile strength

 $f_t(\theta) = f_{t0} - (f_{t0} - f_{t90}) \cdot \frac{|\theta|}{\frac{\pi}{2}} + \left(f_{tmax} - \frac{f_{t1} + f_{t2}}{2}\right) \cdot \sin(4\theta)$ with  $f_{tmax} = \sqrt{f_{t0}^2 + f_{t90}^2}$ 

#### Post-peak behaviour

### **Compressive Behaviour**

$$f_c(\theta) = f_{c0} + (f_{c90} - f_{c0}) \cdot \frac{|\theta|}{\frac{\pi}{2}}$$

$$G_{fc}(\theta) = \alpha_c + (G_{fc0} - G_{fc90}) \frac{|f_c(\theta)|}{|f_{c0}| - |f_{c90}|}$$

with 
$$\alpha_c = \frac{(G_{fc90} \cdot |f_{c0}| - G_{fc0} \cdot |f_{c90}|)}{|f_{c0} - f_{c90}|}$$

