



Bezwijkanalyse kokerbrug met 3D schalenmodel (2)

Project in opdracht van RWS

Chantal Frissen
TNO DIANA B.V.

DOV - 25 november 2010 1



Inhoud presentatie

- Introductie
- Geometrie
- Elementenverdeling
- Materiaal eigenschappen
- Resultaten
- Conclusies

DOV - 25 november 2010 2



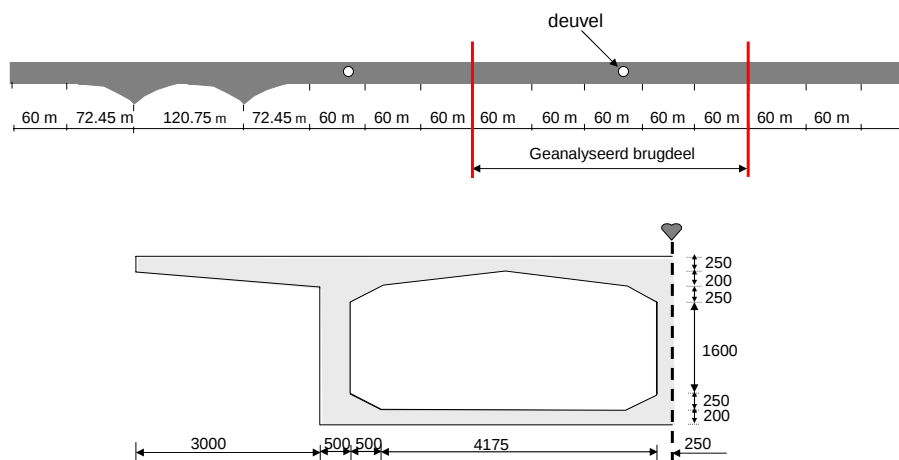
Waarom nieuwe analyse schalenmodel?

- Uit recent onderzoek met 3D volume model (afstudeerwerk Niels Kostense) bleek dat het gedrag van de oplegblokken grote invloed heeft op de resultaten.
- Na de analyse met het schalenmodel in 2008 zijn materiaalmonsters uit de brug beproefd in het laboratorium.

DOV - 25 november 2010 3



Geometrie kokerbrug

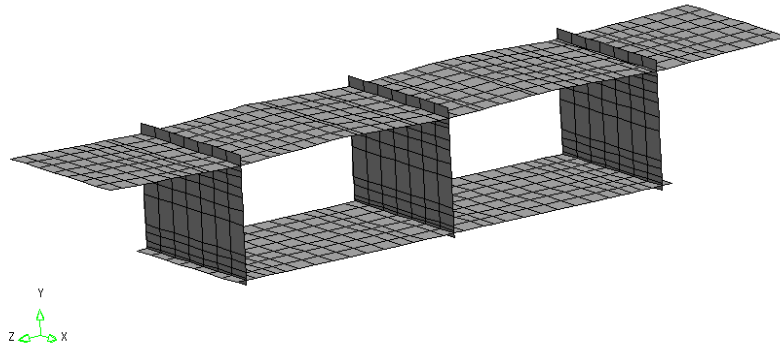


DOV - 25 november 2010 4



3D Schalenmodel

- Bestaand model uit 2008

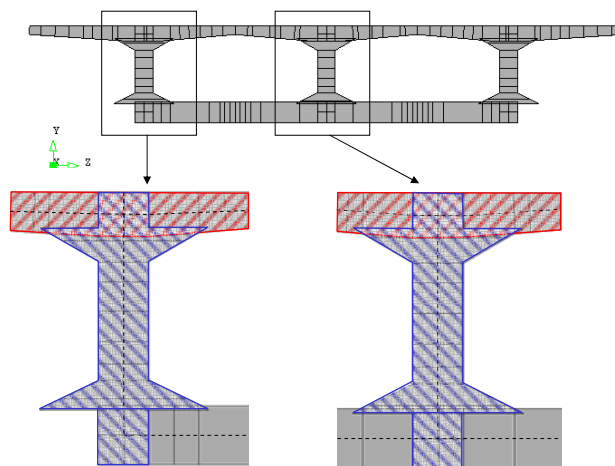


DOV - 25 november 2010

5



Diktes schaalelementen

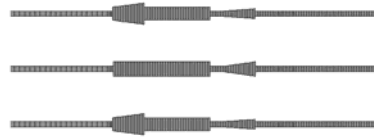


DOV - 25 november 2010

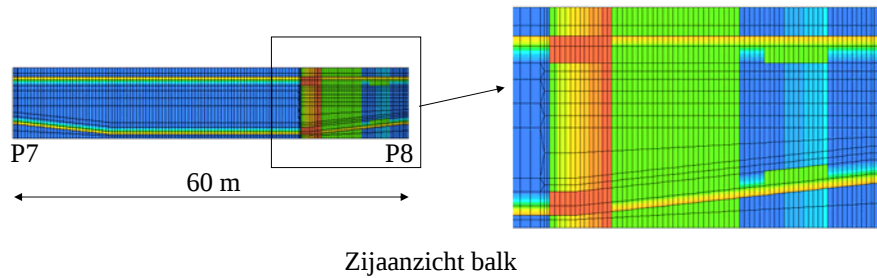
6



Verlopende diktes balken



Bovenaanzicht (alleen de 3 'balken')

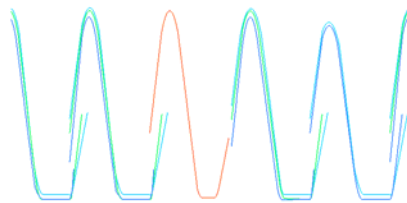


DOV - 25 november 2010 7



Voorspanning in langsrichting

- 43 kabels in de lijven



Zijaanzicht

- 22 kabels in bovenflens bij pijler 10



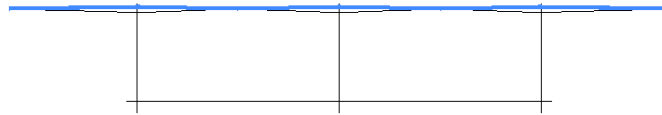
Bovenaanzicht

DOV - 25 november 2010 8



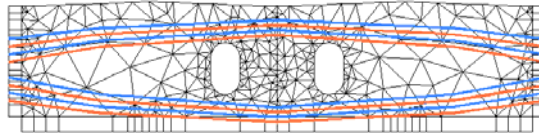
Voorspaning in dwarsrichting

- Bovenflens



Vooraanzicht

- Pijlers



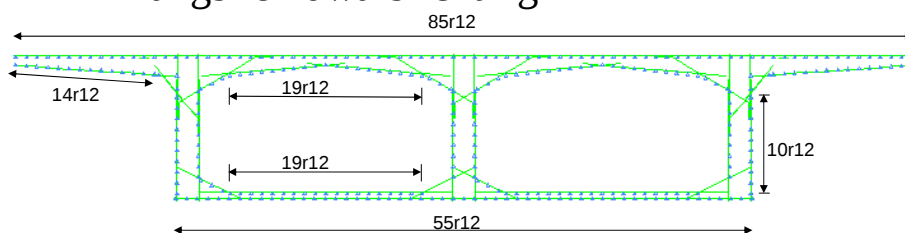
Vooraanzicht

DOV - 25 november 2010 9



Betonstaal

- In langs- en dwarsrichting



- Begin/eind voorspankabels lijven
- Ter plaatse van deuvel en pijlers

DOV - 25 november 2010 10

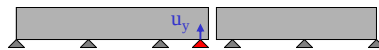


Gefaseerde niet-lineaire analyse

Fasering vanwege bouwwijze kokerbrug

Bouwphase 1:

1. eigen gewicht + voorspanning + u_y tijdelijke ondersteuning



Bouwphase 2

1. Eigen gewicht + voorspanning + deuvel
2. + rustende belasting
3. + asfaltbelasting
4. + 5 rijstroken Qmobiele belasting Eurocode
5. + Pmobiel Eurocode
6. + Qmobiel stapsgewijs verhogen tot bezwijken



DOV - 25 november 2010 11



Niet-lineaire materiaalgedrag

Kokerligger, beton (B55)

Elasticiteitsmodulus: $E = 33077 \text{ N/mm}^2$
Dwarsconstractie: $\nu = 0.15$
Volumieke massa: $\gamma = 25.0 \cdot 10^{-6} \text{ N/mm}^3$
(inclusief wapening)

“Total-Strain-Rotated-Crack”- model

Treksterkte: $f_{bm} = 3.18 \text{ N/mm}^2$

(in 2008: $f_{bm} = 2.74 \text{ N/mm}^2$)

Lineaire softening

Druksterkte: $f'_b = 42.75 \text{ N/mm}^2$

(in 2008: $f'_b = 20.0 \text{ N/mm}^2$)

Ideaal plastisch

Oplegblokken

Niet-lineaire gedrag: Alleen druk
(in 2008: Lineair elastisch)

Staal

Elasticiteitsmodulus: $E = 200000 \text{ N/mm}^2$
Dwarsconstractie: $\nu = 0.3$
Volumieke massa: $\gamma = 76.5 \cdot 10^{-6} \text{ N/mm}^3$

Voorspanstaal (FeP 1860)

Vloeispanning: $f_{pu} = 1367 \text{ N/mm}^2$

(in 2008: $f_{pu} = 1674 \text{ N/mm}^2$)

Ideaal plastisch volgens Von Mises met ‘hardening’:

$f_{y1} = 1367 \text{ N/mm}^2$

$e1 = 0.0$

$f_{y2} = 1519 \text{ N/mm}^2$

$e2 = 0.035$

Voorspanstaal (FeP 835/1030)

Vloeispanning: $f_{pu} = 682 \text{ N/mm}^2$

(in 2008: $f_{pu} = 758 \text{ N/mm}^2$)

Ideaal plastisch volgens Von Mises met ‘hardening’:

Betonstaal (FeB 500)

Vloeispanning: $f_y = 333 \text{ N/mm}^2$

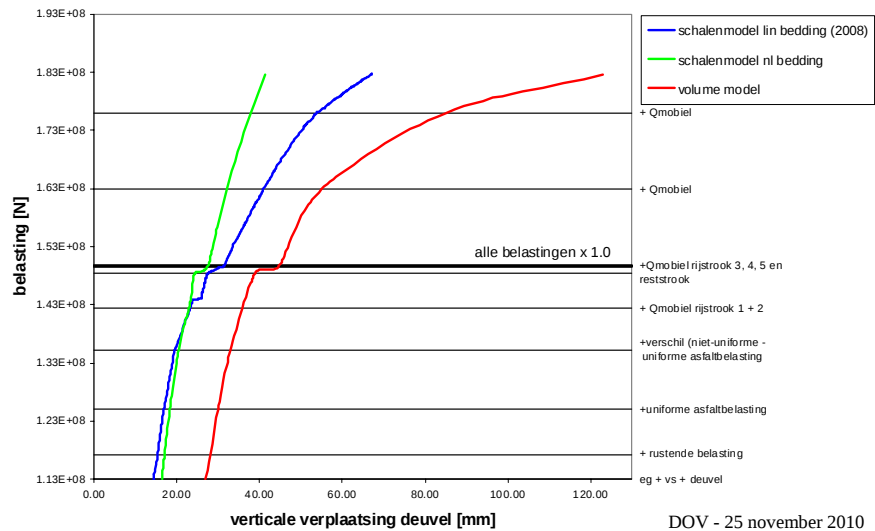
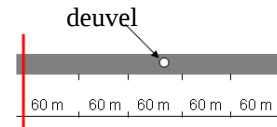
(in 2008: $f_y = 400 \text{ N/mm}^2$)

Ideaal plastisch volgens Von Mises

DOV - 25 november 2010 12



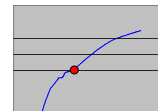
Last-verplaatsing diagram



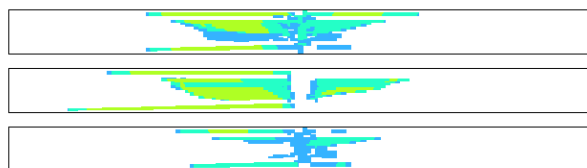
DOV - 25 november 2010 13



1.0 x alle belastingen



2008



.1E-1
.5E-2
.1E-2
.5E-3
.1E-3

Max scheurrek = $0.462 \cdot 10^{-2}$, Scheurwijdte = 1.2 mm

2010



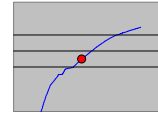
.1E-1
.5E-2
.1E-2
.5E-3
.1E-3

Max scheurrek = $0.297 \cdot 10^{-3}$, Scheurwijdte = 0.07 mm

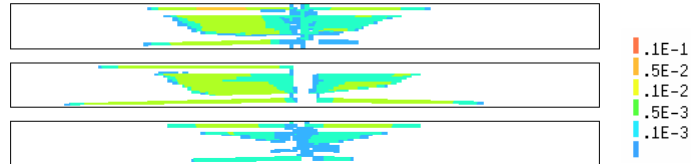
DOV - 25 november 2010 14



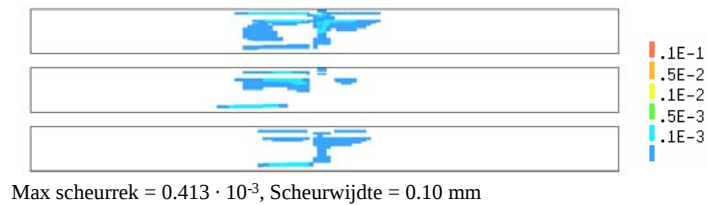
1.5 x Qmobiele belasting



2008



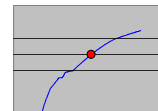
2010



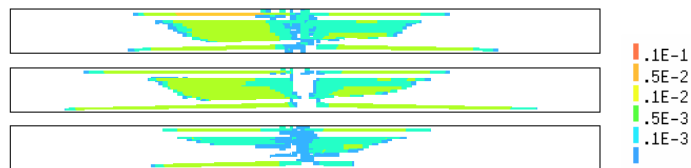
DOV - 25 november 2010 15



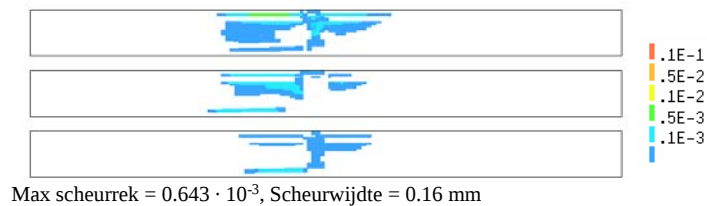
2.0 x Qmobiele belasting



2008



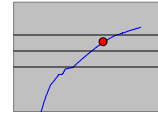
2010



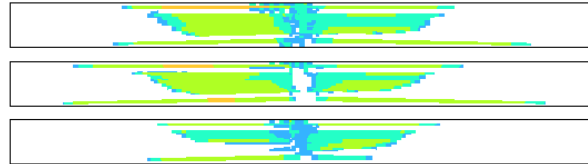
DOV - 25 november 2010 16



2.5 x Qmobiele belasting



2008



.1E-1
.5E-2
.1E-2
.5E-3
.1E-3

Max scheurrek = $0.955 \cdot 10^{-2}$, Scheurwijdte = 2.4 mm

2010



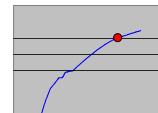
.1E-1
.5E-2
.1E-2
.5E-3
.1E-3

Max scheurrek = $0.952 \cdot 10^{-3}$, Scheurwijdte = 0.24 mm

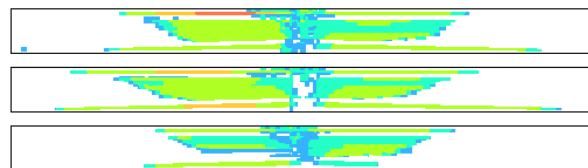
DOV - 25 november 2010 17



3.0 x Qmobiele belasting



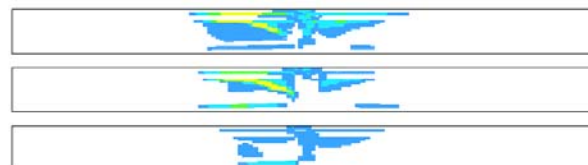
2008



.1E-1
.5E-2
.1E-2
.5E-3
.1E-3

Max scheurrek = $1.160 \cdot 10^{-2}$, Scheurwijdte = 2.9 mm

2010



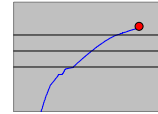
.1E-1
.5E-2
.1E-2
.5E-3
.1E-3

Max scheurrek = $0.480 \cdot 10^{-2}$, Scheurwijdte = 1.20 mm

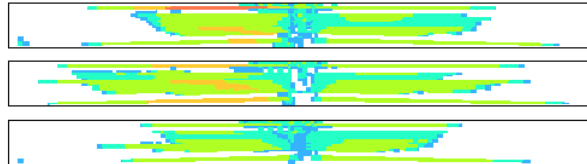
DOV - 25 november 2010 18



3.5 x Qmobilele belasting



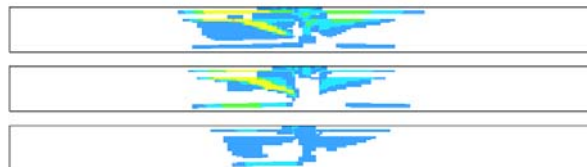
2008



.1E-1
.5E-2
.1E-2
.5E-3
.1E-3

Max scheurrek = $1.580 \cdot 10^{-2}$, Scheurwijdte = 4.0 mm

2010



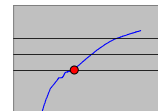
.1E-1
.5E-2
.1E-2
.5E-3
.1E-3

Max scheurrek = $0.505 \cdot 10^{-2}$, Scheurwijdte = 1.26 mm

DOV - 25 november 2010 19



1.0 x alle belastingen



.1E-1
.5E-2
.1E-2
.5E-3
.1E-3

Scheurrek max = $0.297 \cdot 10^{-3}$, scheurwijdte = 0.07 mm



-.6E-3
-.35E-2

Minimale hoofddrekken (druk) beton min = -0.97‰



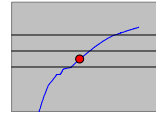
.2E-2
.167E-2
-.167E-2
-.2E-2

Totale rekken ϵ_{YY} in wapening grid Y, min = -0.07%, max = 0.04%

DOV - 25 november 2010 20



1.5 x Qmobiel



Scheurrek max = $0.413 \cdot 10^{-3}$, scheurwijdte = 0.10 mm

.1E-1
.5E-2
.1E-2
.5E-3
.1E-3



Minimale hoofdrekken (druk) beton min = -1.01‰

-.6E-3
-.35E-2



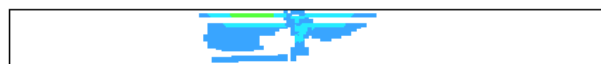
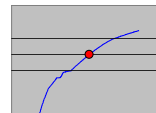
Totale rekken ϵ_{yy} in wapening grid Y, min = -0.07%, max = 0.04%

.2E-2
.167E-2
-.167E-2
-.2E-2

DOV - 25 november 2010 21



2.0 x Qmobiel



Scheurrek max = $0.643 \cdot 10^{-3}$, scheurwijdte = 0.16 mm

.1E-1
.5E-2
.1E-2
.5E-3
.1E-3



Minimale hoofdrekken (druk) beton min = -1.08‰

-.6E-3
-.35E-2



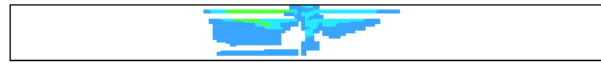
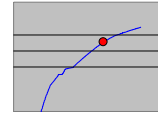
Totale rekken ϵ_{yy} in wapening grid Y, min = -0.08%, max = 0.05%

.2E-2
.167E-2
-.167E-2
-.2E-2

DOV - 25 november 2010 22

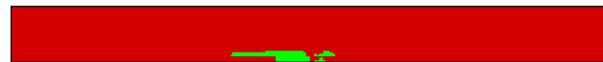


2.5 x Qmobiel



Scheurrek max = $0.952 \cdot 10^{-3}$, scheurwijdte = 0.24 mm

.1E-1
.5E-2
.1E-2
.5E-3
.1E-3



Minimale hoofdrekken (druk) beton min = -1.16‰

-.6E-3
-.35E-2



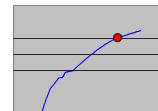
Totale rekken ϵ_{YY} in wapening grid Y, min = -0.08%, max = 0.06%

.2E-2
.167E-2
-.167E-2
-.2E-2

DOV - 25 november 2010 23



3.0 x Qmobiel



Scheurrek max = $0.480 \cdot 10^{-2}$, scheurwijdte = 1.20 mm

.1E-1
.5E-2
.1E-2
.5E-3
.1E-3



Minimale hoofdrekken (druk) beton min = -1.26‰

-.6E-3
-.35E-2



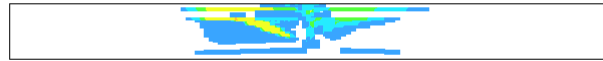
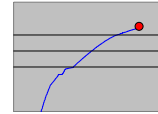
Totale rekken ϵ_{YY} in wapening grid Y, min = -0.08%, max = 0.31%

.2E-2
.167E-2
-.167E-2
-.2E-2

DOV - 25 november 2010 24



3.5 x Qmobiel



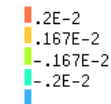
Scheurrek max = $0.505 \cdot 10^{-2}$, scheurwijdte = 1.26 mm



Minimale hoofdreken (druk) beton min = -1.40‰



Totale rekken ε_{YY} in wapening grid Y, min = -0.08%, max = 0.37%



DOV - 25 november 2010 25



Conclusies

- Door verbeteringen in DIANA en door de hogere trek- en druksterkte duurt de analyse nu ongeveer 6 dagen (in 2008 ongeveer 3 weken)
- De hogere treksterkte zorgt voor een stijver gedrag van de constructie. De maximale verticale verplaatsing wordt gereduceerd met 39%.
- Het niet-lineair gedrag van de oplegblokken heeft bij deze modellering van het schalenmodel weinig effect op de verticale verplaatsingen ter hoogte van de deuvel.
- Er treedt geen plasticiteit op in de voorspankabels
- Het rekenmodel van de constructie kan 3.5 x Qmobiel Eurocode belasting dragen:
 - Max doorbuiging = 44.6 mm
 - scheurwijdte = 1.26 mm,
 - min drukrek. = -1.4‰ in de balken en -3.44‰ in de onderflens bij de oplegging.
 - max voorspanning = 1110 N/mm²,

DOV - 25 november 2010 26