

Toepassing detonatiebelasting volgens Eurocode

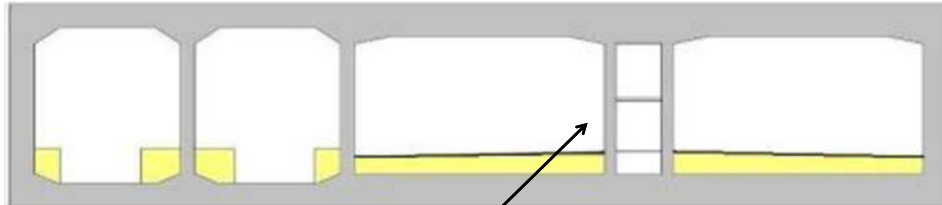
Marcel 't Hart

DOV lezingenavond 29 november 2012

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- Analysis
- Determination of DLF
- Conclusion

Cross-section



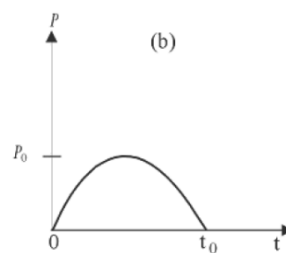
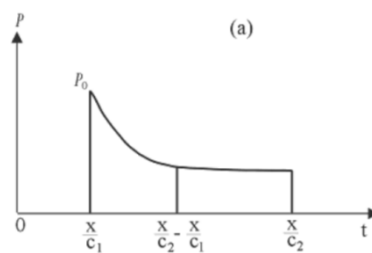
Weak part

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Eurocode

- Detonation - Global
- Deflagration - Local



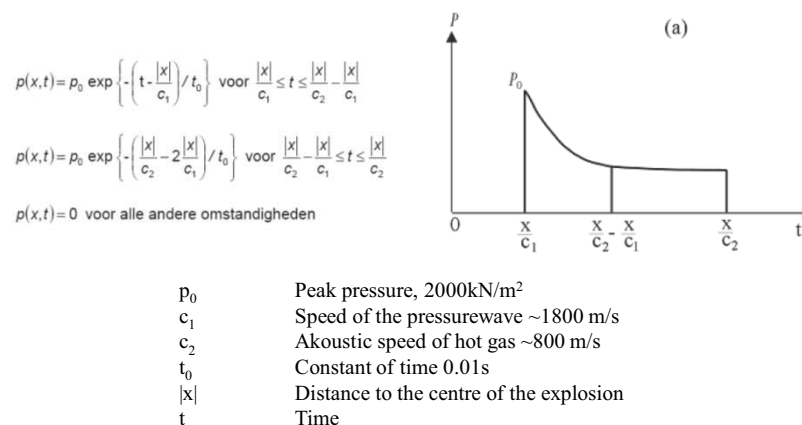
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Causes of a detonation

- Large parts of the tunnel tube are filled with a flammable or highly explosive gas
- Sparkle to ignite
- Combustion will lead to a shockwave which has a speed of sound in the gas, the flamefront is directly coupled to the pressure profile

Detonation in NEN-EN 1991-1-7 D.3



First engineering approach

Apply p_0 as a static load



FAILS



Be smarter, use DIANA



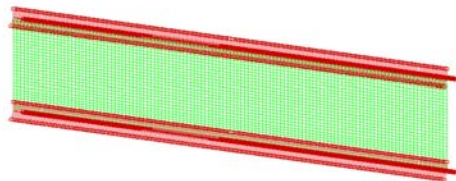
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Model – intermediate wall

Model: WALL_II
Analysis: Static
Model Type: Structural 3D



20000
Quadratic curved shell elements (CQ40S)



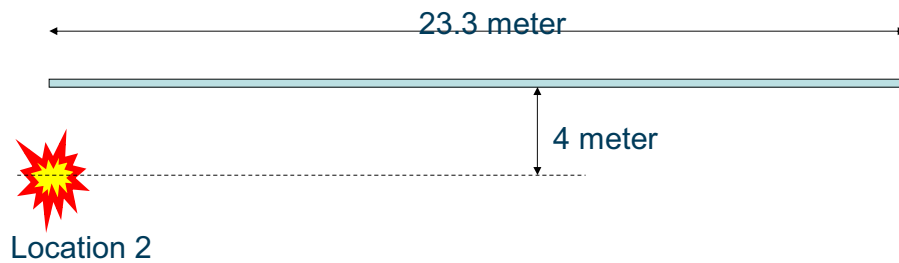
- Clamped supported
- Regular concrete
- $E = 30000\text{MPa}$
- $\nu = 0.15$
- $d = 0.5\text{m}$

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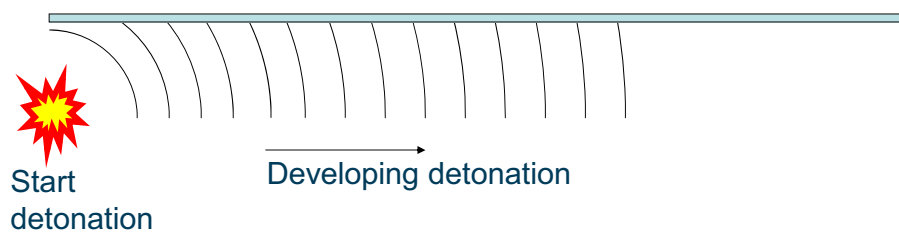
Loading



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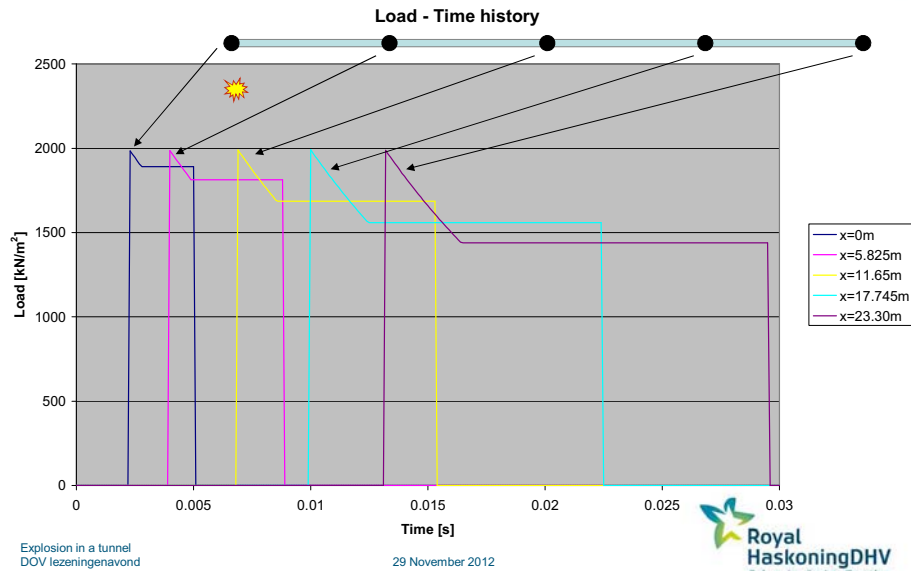
Loading



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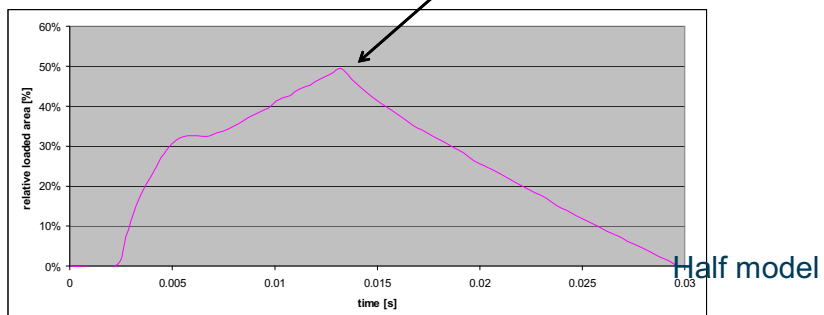
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Load – Time history



Relative loaded Area

$$A = \frac{F_{\text{sum of loads}}}{P_0}$$



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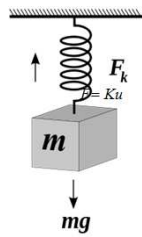
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Solution methods: Static - Dynamic

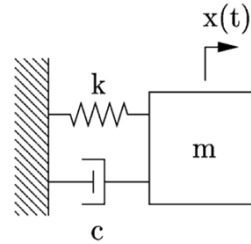
Static

$$F = Ku$$



Dynamic

$$M\ddot{u} + C\dot{u} + Ku = F(t)$$



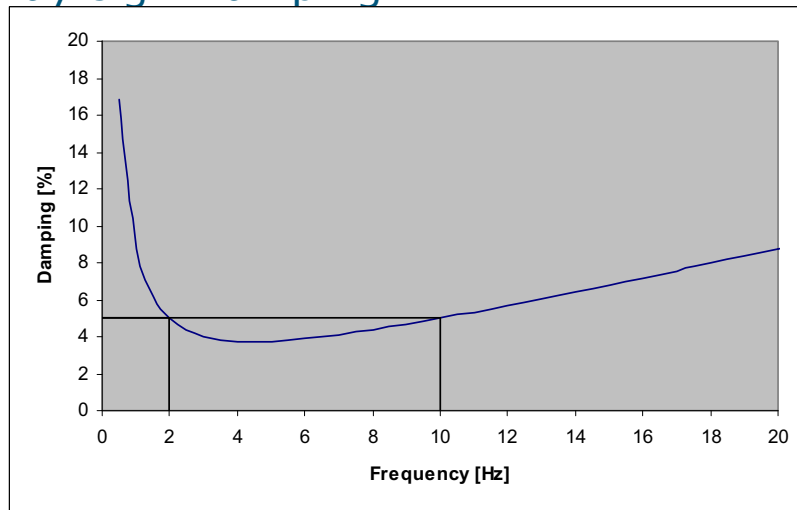
Rayleigh Damping

$$C = aM + bK$$

$$a = 2\omega_1\omega_2 \frac{\omega_1\xi_2 - \omega_2\xi_1}{\xi_2^2 - \xi_1^2}$$

$$b = 2 \frac{\omega_1\xi_1 - \omega_2\xi_2}{\xi_2^2 - \xi_1^2}$$

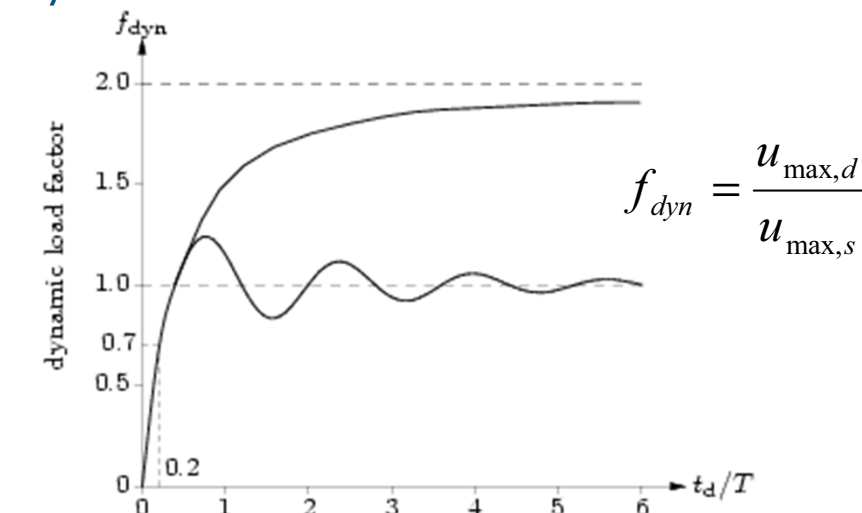
Rayleigh Damping



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Dynamic Load Factor

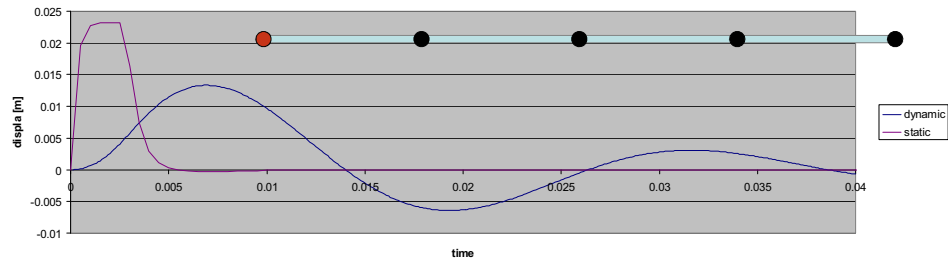


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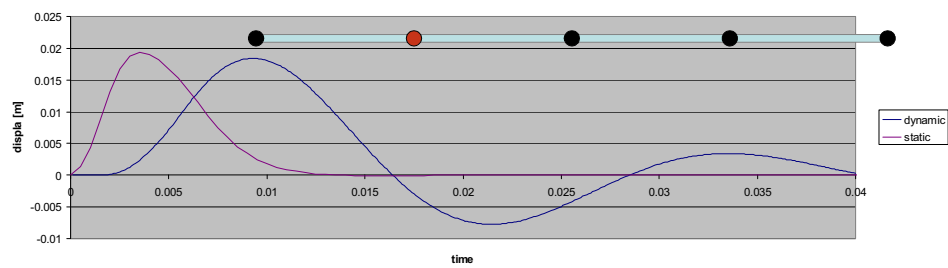
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Displacements

X=0

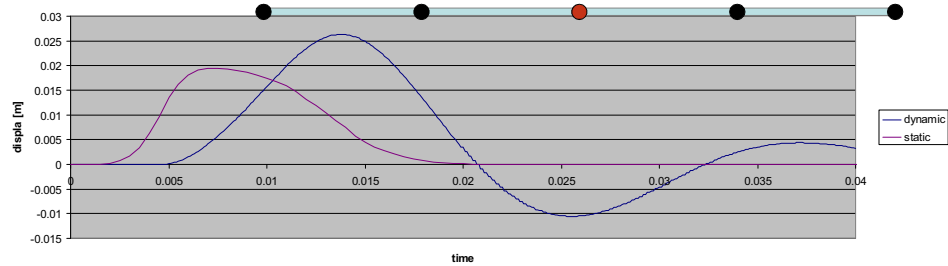


X=5.825m

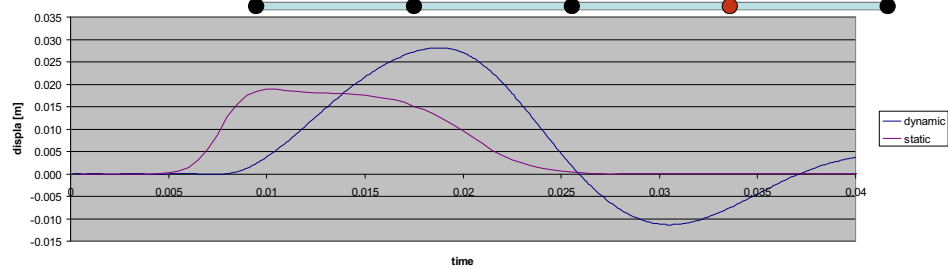


Displacements

X=11.15m



X=17.475m



Dynamic Load Factor

Point	DLF
1	0.57
2	0.95
3	1.35
4	1.49
5	1.29

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Conclusion

If the load of an detonation has to be applied on the tunnel, large areas need to be loaded with $2000\text{MPa} \times \text{DLF}$, which will result in a failure of structure in the Ultimate Limit State. Significant strengthening of the structure is needed to resist this load

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Conclusion

You do not want a tunnel if you want to resist this load



3 recommendations left:

- No tunnel
- Heavier tunnel
- Exclude this load using a probabilistic approach

Be more smart

Use DIANA non-linear

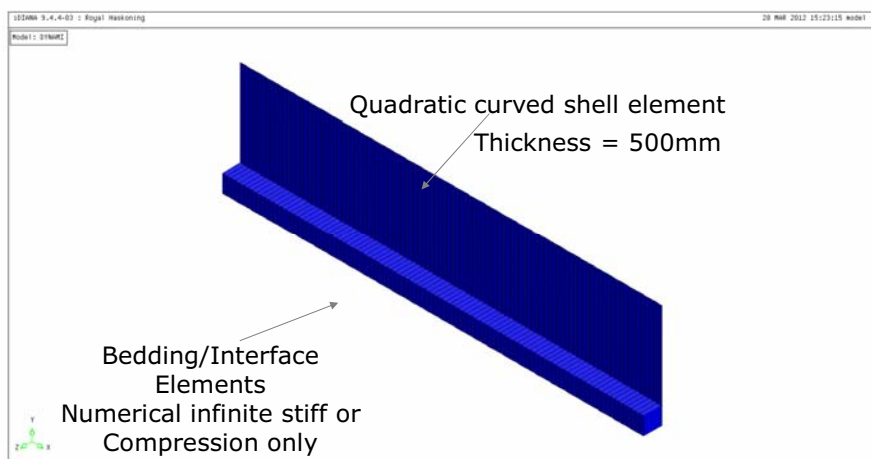
Starting points

- Nonlinear dynamic approach
- Concrete nonlinear
- Reinforcement nonlinear
- Damage may occur, but must be repairable
- Draw a conclusion

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Model



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Reinforcement

Vertical

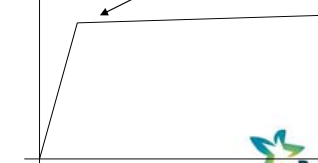
Configuration	Area	Length over height
$\phi 25-200 + \phi 32-200$	6476mm ² /m	1.5m
$\phi 25-200 + \phi 20-200$	4025mm ² /m	1.9m
$\phi 25-100$	4909mm ² /m	1.4m

$f_{yd} = 500 \text{ MPa}$
 $E = 210000 \text{ MPa}$
 $\rho = 7850 \text{ kg/m}^3$

Horizontal

Configuration	Area	Length over height
$\phi 16-150$	1340mm ² /m	2.0m
$\phi 16-225$	894mm ² /m	3.8m

σ
 Small slope (numerical) $E/1000$



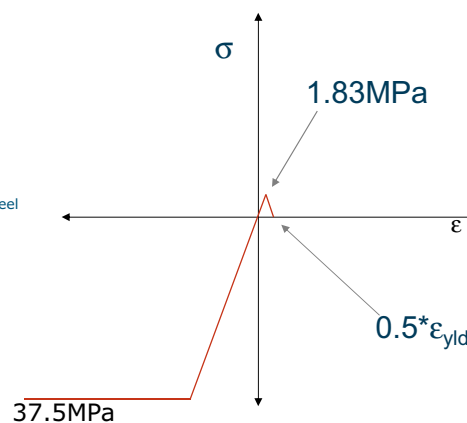
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Concrete

- $E = 34000 \text{ MPa}$
- Smeared rotating crack model
- Tensile strength = 1.83MPa
- Compressive strength = 37.5 MPa
- Max. tensile strain = $0.5 \cdot \epsilon_{\text{yldsteel}}$
- Constant shear retention $\beta = 0.01$
- $\rho = 2500 \text{ kg/m}^3$



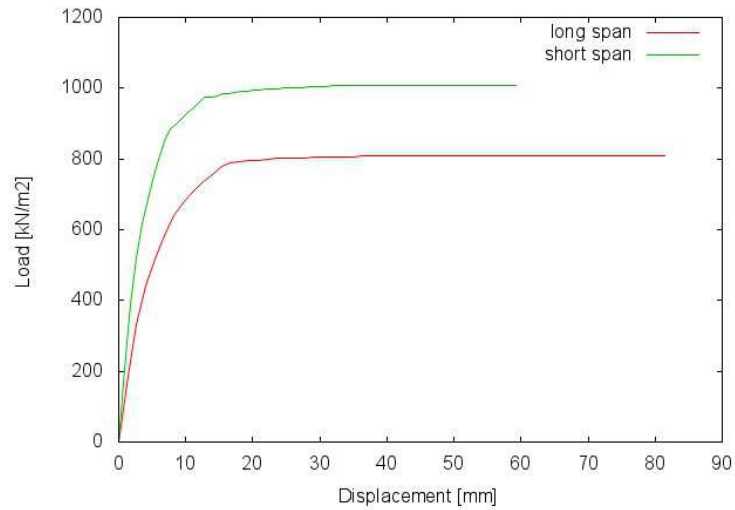
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Influence on the bedding

force - displacement diagram



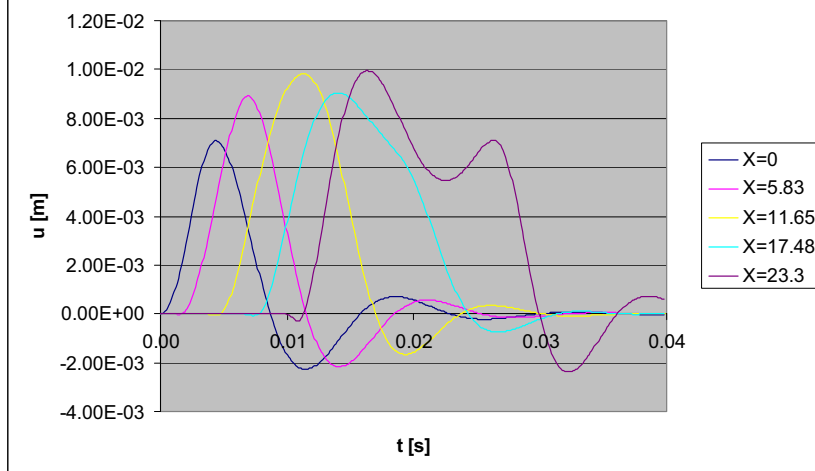
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Linear dynamic

Linear dynamic

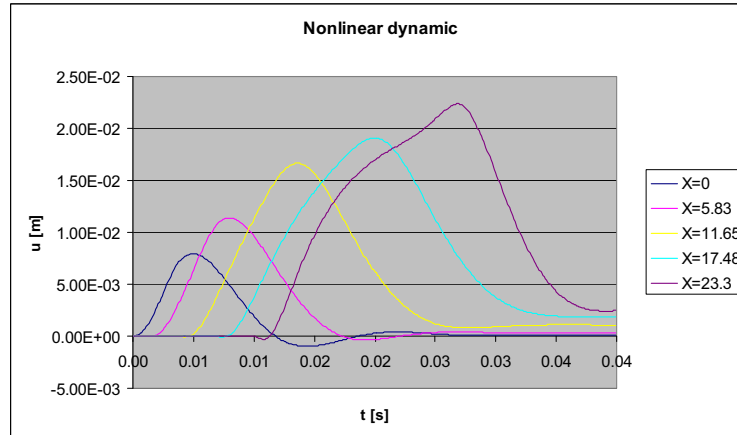


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Nonlinear dynamic

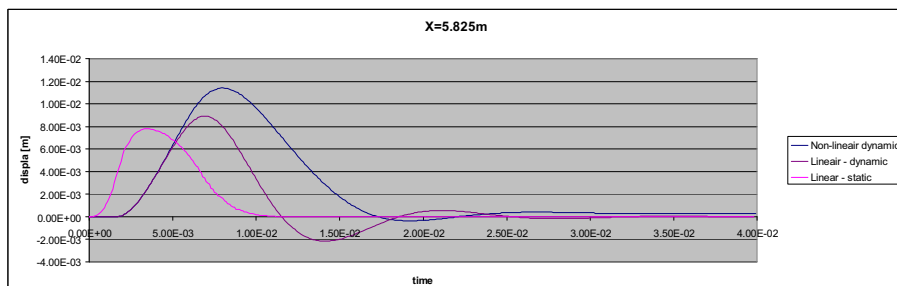
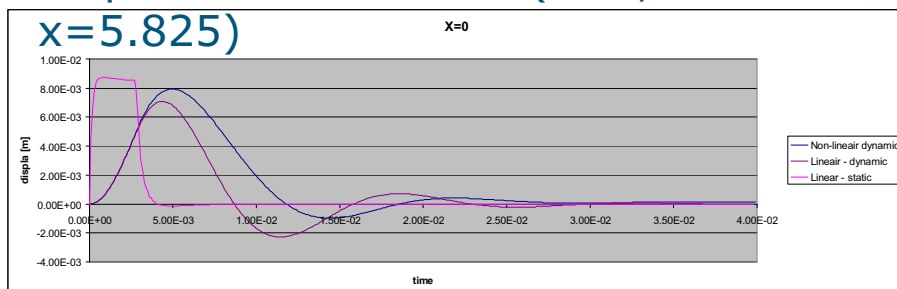


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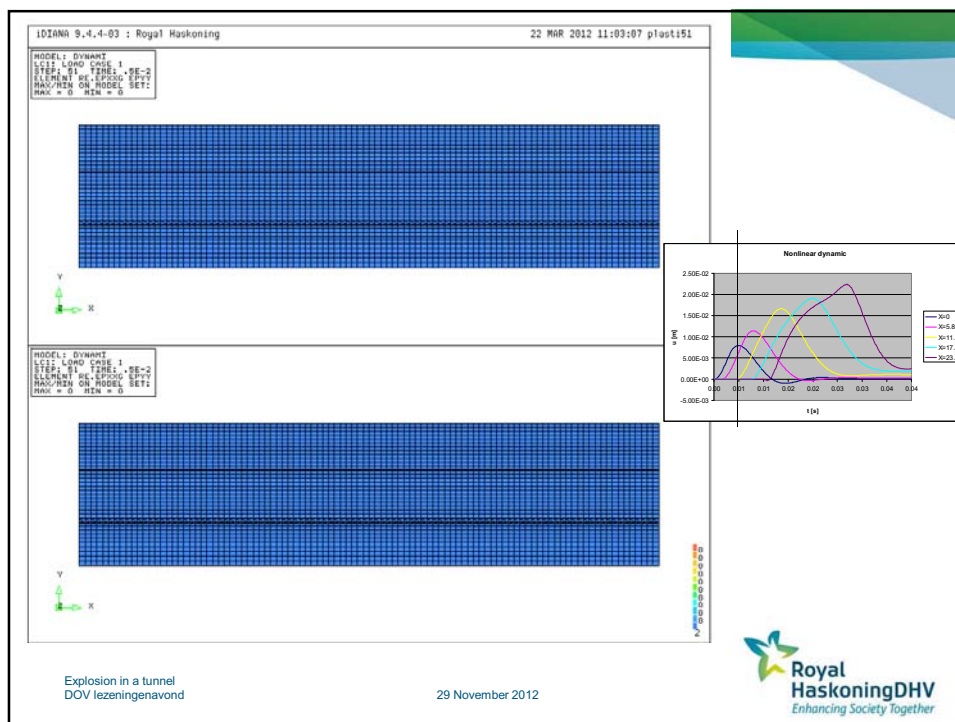
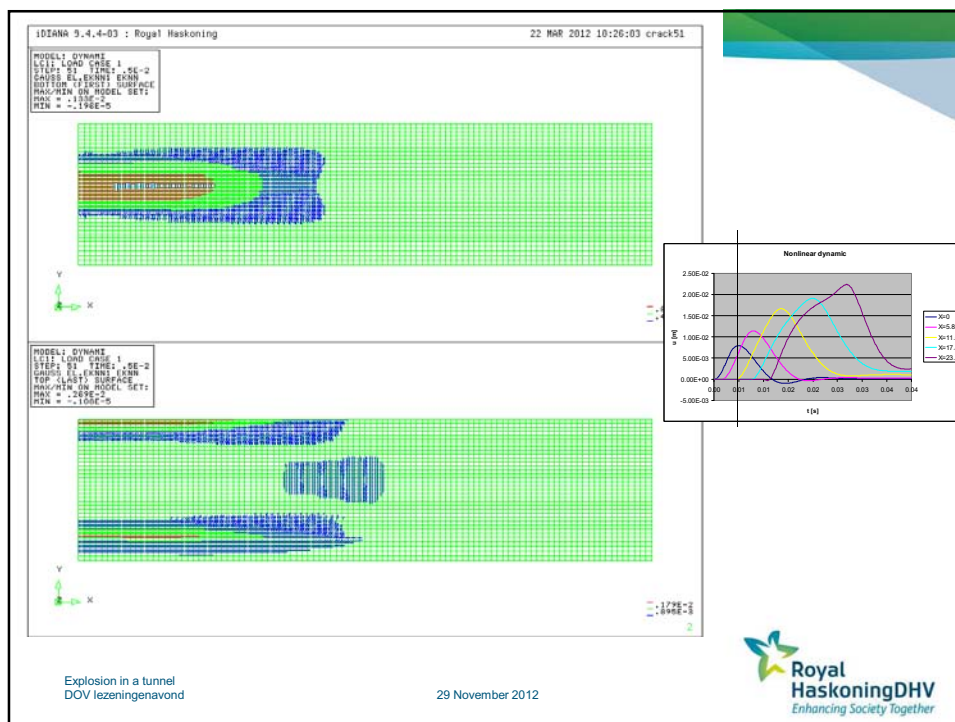
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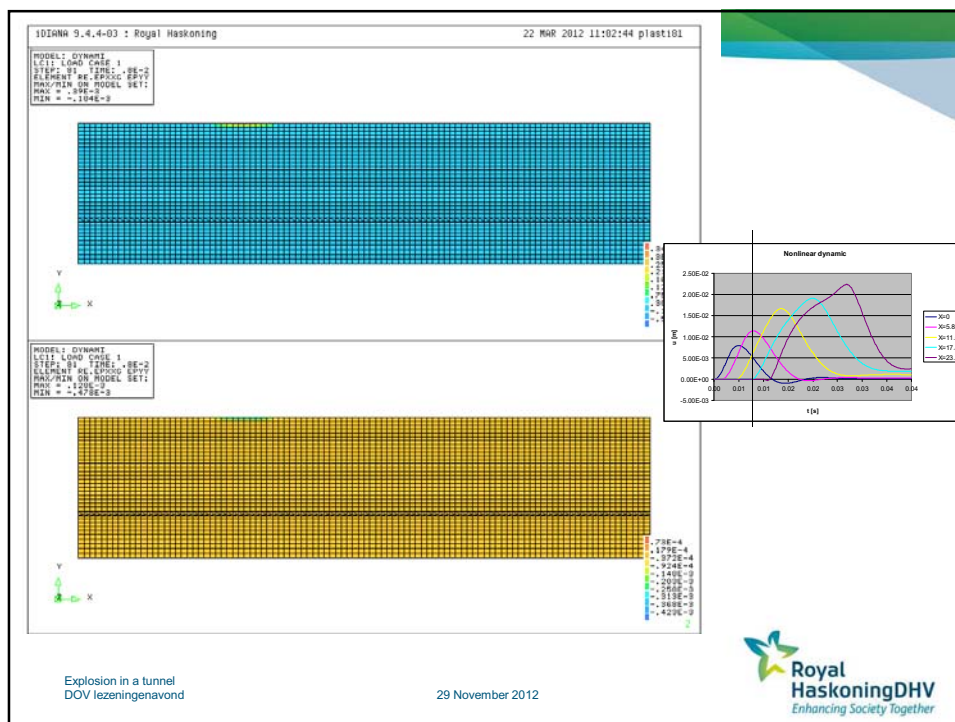
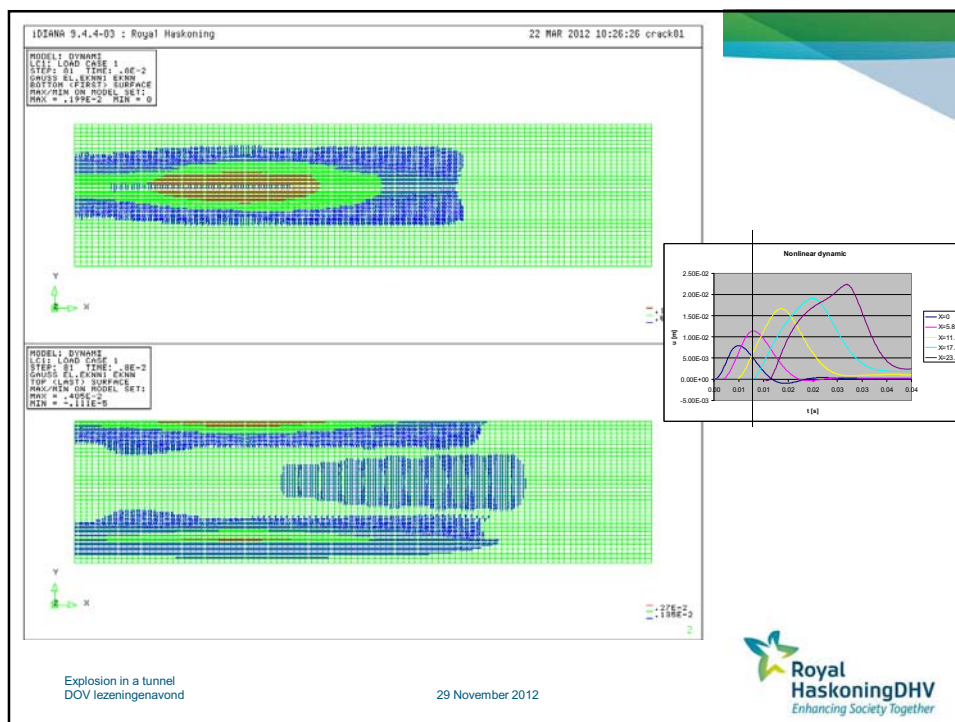


Displacements in time ($x=0$; $x=5.825$)

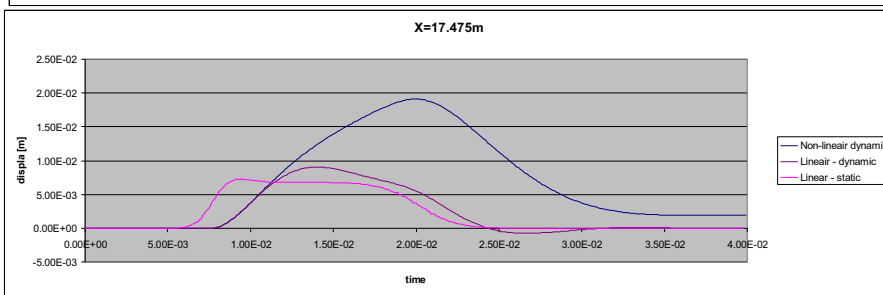


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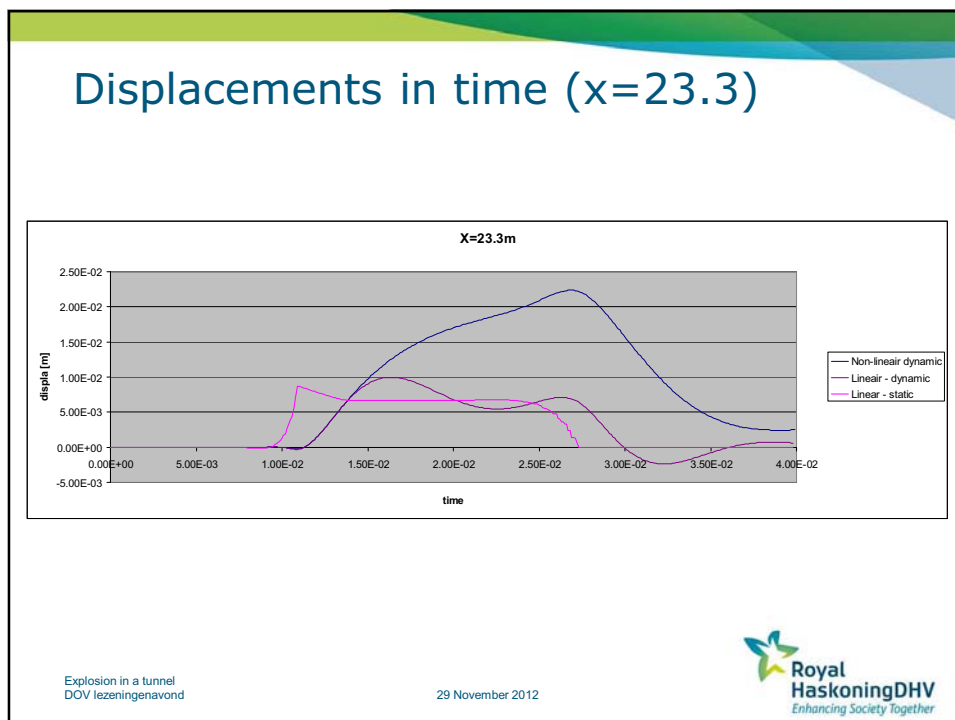
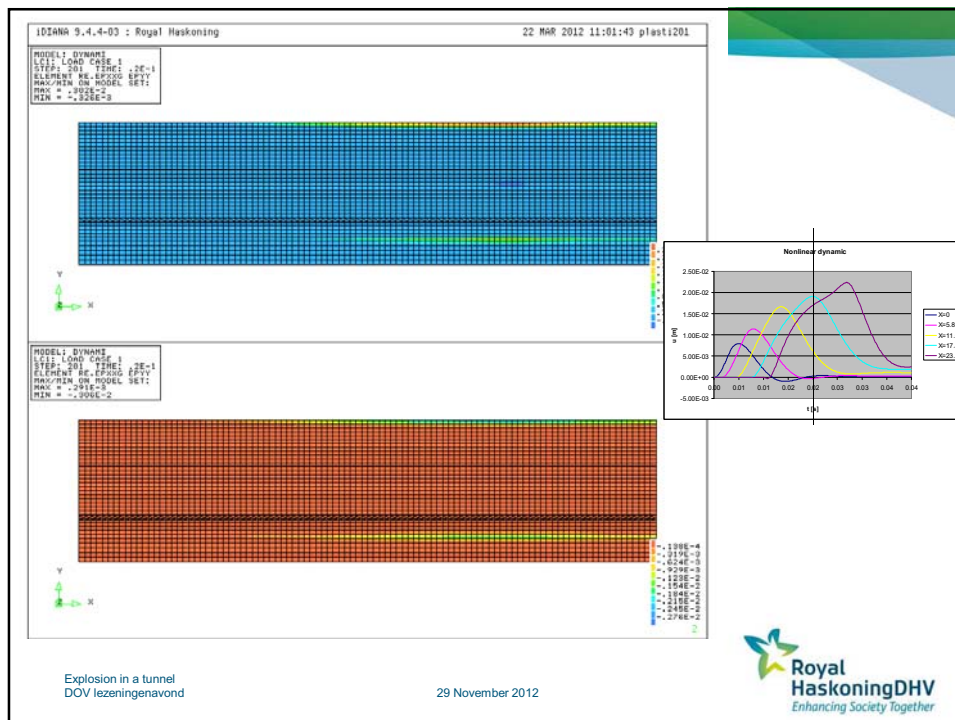


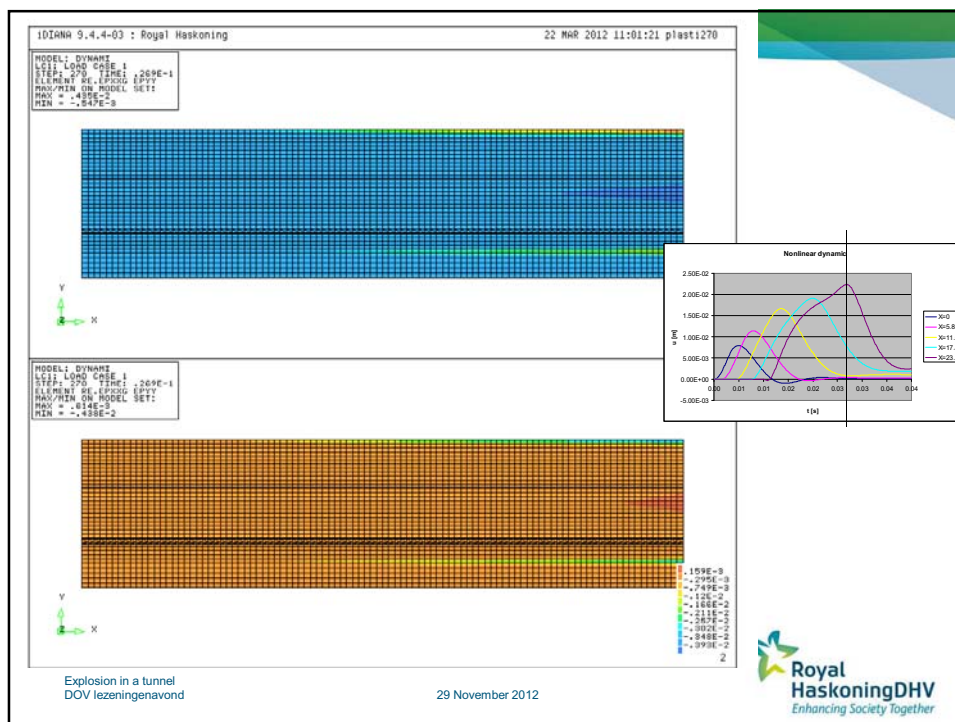
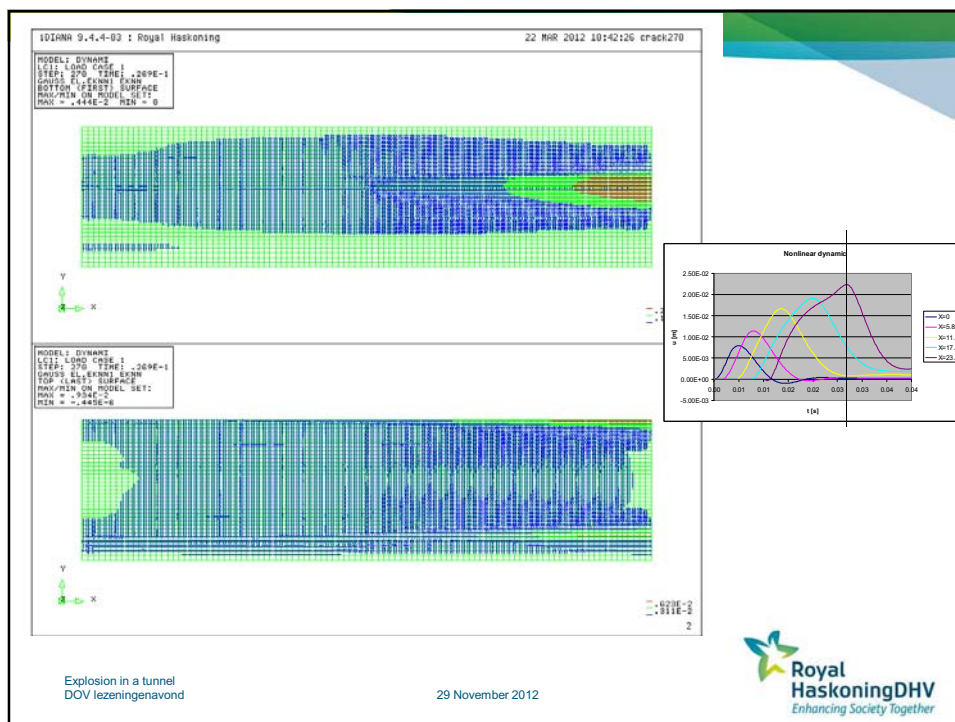
X=11.15m



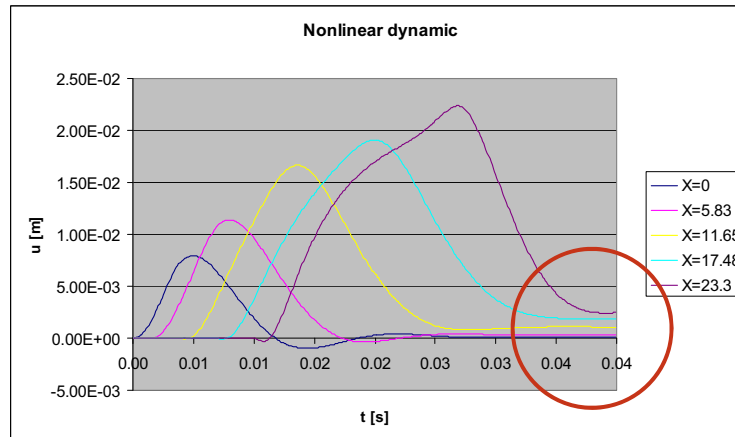
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Nonlinear dynamic



Plastic deformation

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Conclusions

- Wall can handle the dynamic load
- Only minor plastic deformations
- Substantial damage, but repairable

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